#### **Review Article**

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# Mechanical performance of concrete made with sewage sludge ash: A review (Part I)

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Abstract: Sewage sludge is frequently stable and safe when used as construction materials since it bypasses several of the costly and energy-intensive phases of usage. This is supported by numerous studies, particularly when the proportion of sewage sludge is at ideal levels. The primary goal of this article is to demonstrate the use of sewage sludge in building and construction materials. Novel properties such as slump flow and setting time as well as strength properties such as compressive strength, tensile strength, flexural strength, elastic modulus, impact energy, and absorbed energy are the main aspects studied in this review (Part I). Part II describes the physical and chemical properties, durability, and morphological structure of the sewage sludge ash. The findings in this review (Part I) indicate that the flow of concrete decreased with the addition of sewage sludge ash due to its porous nature, but the strength properties improved with sewage sludge ash due to micro-filling voids and pozzolanic activity, which improved the binding properties of cement paste. However, optimum substitution is critical as a greater proportion decreased the strength due to a deficiency of flowability, which boosted compaction energy leading to more cavities. Different studies recommend different optimum doses. However, the typical range for the optimum dose of sewage sludge ash is 5-10% (by wt) of cement.

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#### 1 Introduction

More energy and raw materials are used in current construction industry. Around the world, concrete is a widely utilized building material [1-3]. An energy-intensive method is used to synthesize cement, a key ingredient in concrete. Many greenhouse gas emissions from cement production cause climate change [4-6]. With a contribution of 5-7% to overall CO<sub>2</sub> emissions, cement manufacturers are the second-largest industrial CO<sub>2</sub> emitter [7,8]. To reduce  $CO_2$ , the use of different waste products as cement alternatives has been explored [9-11]. In modern societies, the use of waste as a secondary material in building engineering is a practical and sustainable way to get rid of waste [12-15]. The importance of sustainability in the building sector has increased, and much work has been carried out to reduce the ecological impact of the current building activities [16-18]. Growing energy supply costs, declining CO<sub>2</sub> discharges, and the supply of unrefined, low-quality materials are all harmful to the cement industry [19,20].

Concrete made from waste materials like plastic trash is one of the solutions to waste disposal, according to Faraj et al. [21]. Additionally, the majority of researchers in this century are focusing on creating sustainable concrete by adding different industrial wastes such as corn cob ash [22], sea sand [23], copper slag [24], etc. Sewage sludge ash is another excellent alternative to using diverse waste products as cementitious ingredients in concrete.

The possibility of integrating different waste materials into the manufacture of concrete has drawn greater attention due to the increasing focus on environmental preservation and sustainable growth in the building sector [25–31]. Due to its increasing production, excess sewage sludge in municipal or industrial wastewater treatment plants becomes a significant issue. This growth is due to

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the creation of new industrial units, the expansion of sewage infrastructure, and the advancement of treatment technology. Currently, treating sewage sludge as a substrate provides the foundation for the application and advancement of new technologies connected to its exploitation. Sewage sludge and other waste products may be utilized in the construction business. The finished product is often stable and safe since several of the costly and energy-intensive phases of consumption are eliminated when sewage sludge is utilized in building materials [32].

The management of sewage sludge is an issue not only in European nations but also in Asia. The output of sewage sludge in Japan increased by as much as 170% between 1990 and 2004. Currently, Japan produces more than 2.2 million tons of dry sediments [33]. The progressive reduction in land-fill "capacity" and the EU-wide ban on sewage sludge storage have led to a propensity for people to reject traditional remedies. Additionally, there is a greater focus on the sewage sludge's quality and the compost made from it, which must be regulated in terms of, among other things, the amount of heavy metals and pathogens. Sewage sludge from several European nations does not fulfill the standards for its purity; thus, it cannot be utilized in agriculture or stored and can only be burned in sophisticated incineration facilities [34].

The construction sector, which plays an important role in developing nations, is one of the sectors that may provide the opportunity to utilize waste materials [35–39]. Waste materials from buildings and other sectors include ashes from biofuel incineration, blast furnace slag, sewage sludge, ashes after thermal usage, waste

glass, and materials after building demolition. These factors include pro-ecological activity, sustainable building procedures, and increasing demand for cement [40]. By incorporating sewage sludge into mortars or building supplies, some of the costly and resource-intensive steps in its disposal are cut out. Additionally, the trash that is damaging the environment is changed into a reliable product.

This article provides a review of sewage sludge ash as a concrete ingredient. The important points covered in this study include novel characteristics like slump flow and setting time as well as strength properties such as compressive strength (CPS), tensile strength, flexural strength, elastic modulus, impact energy, and absorbed energy. The results show that the porous nature of sewage sludge ash caused a reduction in the concrete flow but increased the strength properties by micro-filling gaps and pozzolanic activity, which gives more dense mass and improved the cement paste's adhesion.

# 2 Novel properties

## 2.1 Setting time

Figure 1 shows the setting times for different replacement percentages of ash and sewage sludge contents. It should be noted that the setting time was increased using

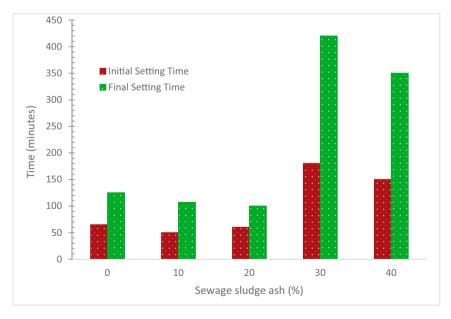


Figure 1: Setting time [41].

sewage sludge ash. Increased water needs are caused by the irregular morphology of sewage sludge's ash and porosity, which is seen from the lesser workability achieved and, in turn, the shorter setting time. The longer setting time is caused by the weaker pozzolanic activity of sewage sludge ash when compared to cement [41].

However, as demonstrated in Figure 1, increased setting time was observed at larger replacement ratios of sewage sludge ash. Because ash is less reactive than cement, cement takes longer to hydrate, which extends the setting time and increases the concentration of the pozzolanic material. The delay in setting time may sometimes be helpful for civil engineering purposes, such as casting deep wells and in certain repair concrete operations [42]. The final setting time is reduced from 345 to 300 min for mortars devoid of red mud and those that contain 20% waste. This influence on the mud may be explained by the presence of aluminum and sodium hydroxides, which are recognized as accelerators [43], as well as by the mud's high alkaline content. The fineness of waste particles might perhaps aid in water retention by outcompeting cement. The remaining free fraction, which may be mixed with cement particles, will be swiftly consumed since all formulations include the same quantity of water [44]. The gap between the setting has also reduced as the wooden ash content of the paste increased from 10 to 30% [45]. Due to a proportionally lower hydration heat of the combined cement as a result of the previously mentioned lowered hydration rate, ash-mixed cement paste was found to be suitable for use. Temperature fluctuations, such as those that occur during large concreting operations, might induce stress but they are mitigated by the needed low-heat development.

#### 2.2 Slump flow

Figure 2 illustrates the slump flow replaced with varying amounts of sewage sludge ash. It should be mentioned that using sewage sludge ash increased the slump flow. The fluidity is significantly decreased by adding sewage sludge ash to the binder. Because the sewage sludge ash particles are porous and have an irregular form, they are more likely to absorb water on their surfaces [46]. The fluidity of mortar falls by 4.07% for every 5% increase in the average sewage sludge ash content. This is caused by porous characteristics and uneven shape of sewage sludge ash [47]. The uneven shape may rearrange the sewage sludge ash particles in a mortar and create a lot of gaps, and the porous microstructure promotes water absorption [48]. Additionally, the workability of mortar is

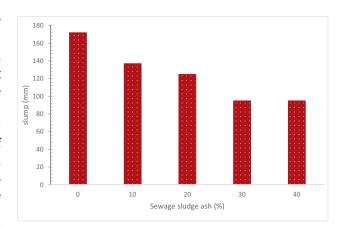


Figure 2: Slump flow [41].

significantly impacted by the sewage sludge ash particle size distribution. The smoother sewage sludge ash particle surface, less interlocking and friction between particles, and improved workability of mortar are all effects of grinding [49].

The slump value for the concrete in both 75 and 150  $\mu$ m particle size decreased as the amount of sewage sludge ash substitution increased [50]. Additionally, it should be noted that the increased surface area of sewage sludge ash particles causes flowability to be negatively impacted by lowering the sewage sludge ash particle size. As a result, more paste is needed to coat the particle, which means that there is less paste available for flowability. Kumar et al. observed that the 75  $\mu$ m drop in slump outweighs the 150  $\mu$ m decrease in the slump flow [50]. Unlike the majority of coal fly ash, the form of sewage sludge ash particles is not spherical. According to previous research, sewage exhibits an uneven grain shape and a noticeably rough surface, as shown in Figure 3.

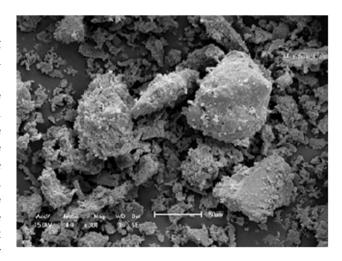


Figure 3: SEM of sewage sludge particles [51].

The workability of mortars containing sewage sludge ash is negatively impacted by this feature (no "lubricant effect" is generated) [52]. A significant amount of water is needed to power hydration operations because the silica fume has a higher specific surface area than cement [53]. The superplasticizer dose was increased to provide the required workability in all silica fume concrete combinations. It should be noted that the flowability of concrete decreased with increasing silica fume substitution. Concrete becomes stiffer and more cohesive and loses flowability when silica fume is substituted with calcium hydroxide because silica interacts with calcium hydroxide when it is in its finely split condition to generate secondary cementitious calcium silicate hydrate (CSH) [54].

# 3 Strength properties

### 3.1 Compressive strength (CPS)

Figure 4 shows the CPS with various replacements of ash and sewage sludge. As opposed to the reference mixes, it should be highlighted that incorporating sewage sludge ash increased the compressive capacity. The research reveals that the 5% sewage sludge ash concrete had the maximum strength of 55.85 MPa at 180 days, 9.94% higher than the control strength, while the 20% sewage sludge ash sample had the lowest strength of 30.36 MPa strength at 180 days. Additionally, it is shown that all of the

samples' strengths increase throughout the curing process. The concrete age, curing technique, and specimen strength are all significantly correlated. In general, concrete samples exposed to the water curing technique have greater overall strengths than concrete samples subjected to the air curing method. This demonstrates how the concrete's ability to build strength has an impact on the curing process. The concrete samples must function as predicted in terms of strength, and the early volume change must be controlled; thus, it is crucial to use the right curing technique [55].

A study used sewage sludge ash, which was obtained directly from the local sewage treatment facility. The water-to-binder ratio (0.55) remained constant while there were five degrees of cement replacement (2.5, 5, 7.5, 10, and 20%). The finding shows that the CPS of mortar samples decreases as the ash content increases. The control sample with no ash from sewage sludge had the maximum average CPS of 35.31 MPa after 7 days of curing. The mortar sample with sewage sludge ash replacing 20% of the cement had the lowest strength of 32.31 MPa. The average CPS of the cement mortar with the addition of 2.5% sewage sludge ash was 35.65 MPa, which was somewhat higher (by 0.95%) than the value of the control mortar. The average CPS of the mortar containing 5, 7.5, 10, and 20% sewage sludge ash decreases by around 2.1, 5.4, 5.9, and 8.5%, respectively, when compared to the control cement mortar [59]. The maximum CPS was shown by the cement replaced with 5% sewage sludge ash at 28 days (33.09 N/mm<sup>2</sup>), while the lowest CPS was demonstrated by the

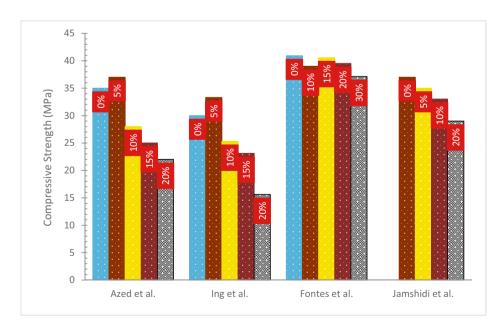


Figure 4: Compressive capacity [55-58].

cement replaced with 20% (15.68 N/mm²) in comparison to control concrete; the substitution of sewage sludge ash at 5% increased the CPS by up to 10% [56]. When cured for 28 days, the CPS of 20 and 30% sludge ash concrete was less than the intended 28-day CPS. However, the CPS of the concrete made with 20% sludge ash was more than the 28-day CPS. This is due to the sludge ash's probable pozzolanic properties, which causes concrete's CPS to decrease at young ages and increase at older ages. Thus, unreinforced concrete might be used with 10% sludge ash concrete. The 20% sludge ash concrete might also be used for non-structural concrete. CPS and workability are not suitable when cement is replaced with a higher dose (30%) of sludge ash [60].

The CPS decreases as the amount of cement replaced by sewage sludge ash increases. The CPS of sewage sludge ash concrete increases with curing time, and the CPS loss is proportional to the percentage increase in sludge ash [58]. Although the maximum benefit was attained for a replacement level of 10–15%, which shows CPS increased by about 74% in comparison to the reference blend, the increase in CPS was also noticeable for replacement levels of 20% (50% more than the reference blend), and 30% (22% more than the reference blend) [57]. Three days of hydration did not reveal any appreciable changes. The concrete mixture with a 5% sludge ash concentration showed nearly no CPS loss from 7 to 28 days of curing. Mixtures that include 10% sludge ash show a 10% decrease in CPS [58].

The CPS of the composite including 20% sludge ash increases with curing time and, after 90 days, approaches that of the mixture containing merely 10% sludge ash. This may be explained by a negligible pozzolanic impact, as suggested by Cyr et al. [61], or by the possibility that sludge ash particles operate as additional sites for the nucleation and development of hydration products, so accelerating the hydration process as a whole [62]. The pozzolanic activity of sewage sludge ash is affected by the increase in the incineration temperature in two different ways. On the one hand, sewage sludge ash contains more amorphous SiO<sub>2</sub> because of the increasing temperature, and this increase in amorphous SiO2 promotes the development of CSH gel in the mortar, thereby improving the mortar performance. On the other hand, the performance of mortar is negatively impacted by the sintering bonding phenomena, which arises because with an increase in temperature, the sewage sludge ash's pozzolanic activity may be somewhat increased with grinding [63]. Table 1 depicts the concrete flowability and strength properties of the concrete with varying proportions of sewage sludge ash.

Figure 5 shows the CPS with varied sewage sludge ash concentrations and curing times. The control or reference concrete (blank mix), CPS after 28 days, was selected as reference strength from which different blend mixes with sewage sludge ash are compared. For comparison, the optimum substitution of sewage sludge ash (5%) was selected. Pozzolanic CPS of 5% sewage sludge ash is less than 15% as compared to the reference strength. However, after 28 and 90 days of cure, the CPS is 6 and 28% more than the reference strength with a 5% substitution of sewage sludge ash. This is because the pozzolanic process proceeds at a much slower pace than the cement hydration process [65], in which materials could have had lower early-age strength than expected. Similar studies arrived at the same result, that is, the addition of pozzolanic components enhanced the concrete strength at a later age (beyond 28 days) [13]. Haustein et al. [59] also reported that the gradual and ongoing reactivity of ash from sewage sludge in cement mortars is responsible for the early-age (7 days) poor CPS value. The findings suggest that sewage sludge ash affects the hydration of cement because of its chemical makeup. It probably has little pozzolanic action and mostly functions as an additive. The sewage sludge ash grains may have trapped a lot of water, and only a small quantity of CH was formed from cement hydration to take part in the pozzolanic interactions with the sewage sludge ash and provide strength to the mortar samples.

## 3.2 Tensile strength

Figure 6 shows the tensile strengths with various binder replacement levels for sewage sludge. It should be highlighted that the tensile strength was not increased while employing sewage sludge ash in comparison to the reference mixes. However, the tensile strength is almost similar to reference mixes up to 5% sewage sludge ash. Furthermore, for high strength, the review recommends the addition of different types of fibers such as jute fiber [66], steel fiber [67], nylon fiber [68], carbon nanofiber [69], glass fiber [70], and basalt fiber [71] to improve its tensile capacity.

Additionally, greater days of curing, better tensile strength, and strength that is similar to the reference mixes up to 5% sewage sludge ash were also reported. The research found that adding sewage sludge ash to polyester dramatically changes the composite's mechanical characteristics. Tensile stress increases in the tensile test results until the weight rate of ash reaches 20%, after which it drops [72]. With up to 5% more sewage sludge

Concrete   Ohio   Ohi	Reference	Reference Concrete type	Sewage sludge ash	Concrete ingredient	Other materials	W/C Slump (mm)	Compre	ssion str	Compression strength (MPa)		Split tensile strength (MPa)	Pa)	Flexure strengt	Flexure strength (MPa)		Remarks
Concrete 0% Cement — — — — — — — — — — — — — — — — — — —	[60]	Conventional		Cement			7.0	28 D	1 94 D	I			ı			Strenoth
Conventional   10%   10%   Conventional   10%   10%   Conventional   10%   Conventional   10%   Conventional   1	2	concrete	%0	, , , ,			, 6	2 77	2 25							decreased
20%   20%			10%				; 2	72	75							3
14   15   17   18   18   18   18   18   18   18			20%				15	19	23							
Conventional         36         76         36         77         80         70         28 D         40 D			30%				14	16	17							
concrete         0%         343         434         435         43         435         43         435         77         43         57         77         43         57         43         435         43	[64]	Conventional		Cement	1	1	3 D	7 D	28 D				7 D	28 D		Slump
9%         55         34         64         35.6         3.5         4.9         5.9           19%         10%         10%         137         15.0         3.7         15.0         3.7         4.9         5.4         4.9         5.4         4.9         5.4         4.0         4.0         15.0         13.7         15.0         13.7         15.0         13.7         15.0         13.7         15.0         13.9         4.1         4.5         5.4         4.7         5.2         4.0         4.0         15.0         4.0         4.0         15.0         4.1         4.5         5.0         4.1         4.5         5.0         5.0         5.1         4.0 <t< td=""><td></td><td>concrete</td><td>%0</td><td></td><td></td><td>20</td><td>31.3</td><td>34.3</td><td>43.9</td><td></td><td></td><td></td><td>5.6</td><td>7.7</td><td></td><td>increased</td></t<>		concrete	%0			20	31.3	34.3	43.9				5.6	7.7		increased
High strength         17.1         2.0.5         17.1         2.0.5         17.1         2.0.5         3.3         4.7         3.4         4.7         5.0         4.7         5.0         10.3         10.3         10.3         10.3         10.3         10.3         10.3         10.3         10.3         10.3         10.3         10.3         10.3         4.7         2.0         2.0         7.0         2.0         7.0         2.0			2%			55	23.4	26.4	35.6				3.8	5.9		and strength
15%   Conventional Convention			10%			55	17.1	20.8	25.3				4.3	5.4		decreased
High strength         Cement         Fly ash         0.38         7D         14 D         28 D         7D         28 D         50 D         7D         36 D         50 D         7D         36 D         40 D         32 D         40 D         40 D         50 D         7D         40 D         50 D         7D         40 D         50 D         40 D         50 D         40 D         50 D         40 D         <			15%			09	13.7	15.0	19.1				3.4	4.7		
tontrete 5% Conventional 5% Co	[20]	High strength		Cement	Fly ash	0.33	7 D	14 D	28 D	1 /			7 D	28 D	26 D	Slump
19%   19%		concrete				180	46.0	53.9	61.1	3.3		4.1	4.5	5.1	5.2	increased
5%   10%			%0			195	43.0	45.9	58.9	3.2		4.0	4.3	5.0	5.1	and strength
10%   10%   10%   10%   10%   15%   15%   14%			2%			162	27.8	36.7	44.7	2.7		3.4	3.5	4.3	4.5	decreased
15%   Additive   Los			10%			157	26.3	37.4	43.9	2.7		3.3	3.4	4.2	4.4	
Conventional         Additive         -         0.50         7 D         28 D         -<			15%			151	42.4	44.6	58.1	2.1		3.1	3.0	3.9	4.1	
concrete         0%         40.92 <th< td=""><td>[57]</td><td>Conventional</td><td></td><td>Additive</td><td>1</td><td></td><td>7 D</td><td></td><td>28 D</td><td>I</td><td></td><td></td><td>ı</td><td></td><td></td><td>7 D strength</td></th<>	[57]	Conventional		Additive	1		7 D		28 D	I			ı			7 D strength
10%   10%		concrete	%0				20.66		40.92							increased
15%   20%   30.53   39.50   30.53   39.50   30.53   39.50   30.53   39.50   30.53			10%				36.01		39.00							and 28 D
20%         30.93         39.50         712         R         26.50         37.12         710         28.0         90 D         710         28.0         710         28.0         710         8.5         8.0         8.5         8.0         8.5         8.0         8.5         8.0         8.5         8.0         8.5         8.0         9.0 <td></td> <td></td> <td>15%</td> <td></td> <td></td> <td></td> <td>35.89</td> <td></td> <td>40.55</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>strength</td>			15%				35.89		40.55							strength
Sometitional   Some			20%				30.93		39.50							decreased
Conventional         Cement         —         0.51 —         7D         28 D         90 D         7D         28 D         90 D           concrete         0%         —         27         35         40         —         64         70         8.5           concrete         5%         —         4         4         5         4         5         7         8         5         8         9			30%				26.50		37.12							
concrete         0%         40         40         40         40         64         70         8.5           10%         30         37         45         65         75         8.0           10%         22         28         32         7         45         7.5         8.0           15%         20%         25         27         4         5         6.5         7.8         8.0           Conventional         30%         40%         40         40         40         4.0         4.0         5.0         7.0         8.0         7.0         8.0         7.0 <td>[55]</td> <td>Conventional</td> <td></td> <td>Cement</td> <td>1</td> <td></td> <td>7 D</td> <td>28 D</td> <td>0 D</td> <td></td> <td></td> <td></td> <td>7 D</td> <td>28 D</td> <td>00 D</td> <td>Strength</td>	[55]	Conventional		Cement	1		7 D	28 D	0 D				7 D	28 D	00 D	Strength
5%       5%       45       45       6.5       7.5       8.0         10%       10%       22       28       32       5.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.0       7.8       6.0       7.8       6.0       7.8       6.3       6.8       7.9		concrete	%0				27	35	40				6.4	7.0	8.5	increased
10%       22       28       32       5.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.8       6.5       7.9       7.8       6.5       7.9       7.8       6.5       7.9       7.8       6.6       7.9       7.8       6.8       7.9<			2%				30	37	45				6.5	7.5	8.0	
15%       20       25       27       5.0       5.8       6.2         20%       18       22       24       4.8       5.5       6.0         Conventional concrete       0%       metakaolin metakaolin       39.3       46.1       54.7       59.4       33       6.3       6.8         20%       metakaolin metakaolin       41.5       53.2       57.5       66.9       31       6.2       7.3         20%       40%       41.5       53.2       57.5       66.9       31       6.2       7.3         40%       40%       40%       42.5       53.3       59.7       2.5       7.3         Conventional concrete       0%       6.5       10       70       28.0			10%				22	28	32				5.8	6.5	7.8	
Conventional       Cement       Silica fume and o.50 - 3D       7D       14D       28D       -       4.8       5.5       6.0         concrete       0%       metakaolin       39.3       46.1       54.7       59.4       -       1D       7D       28D         20%       40%       41.5       53.2       57.5       66.9       3.1       6.3       7.3       6.8         40%       40%       42.5       53.3       59.7       22.5       7.3			15%				20	25	27				2.0	5.8	6.2	
Conventional         Cement         Silica fume and outsolute         0.50         -         3D         7D         14D         28D         -         1D         7D         28D           concrete         0%         metakaolin         39.3         46.1         54.7         59.4         3.3         6.8         6.8           10%         40%         41.5         53.2         57.5         66.9         3.1         6.2         7.3           40%         40%         42.5         53.2         59.7         18.2         5.7         7.3 </td <td></td> <td></td> <td>20%</td> <td></td> <td></td> <td></td> <td>18</td> <td>22</td> <td>24</td> <td></td> <td></td> <td></td> <td>4.8</td> <td>5.5</td> <td>0.9</td> <td></td>			20%				18	22	24				4.8	5.5	0.9	
concrete       0%       metakaolin       39.3       46.1       54.7       59.4       3.3       6.8       6.8         10%       41.5       53.2       57.5       66.9       3.1       6.2       7.3         20%       33.1       42.5       53.2       57.5       66.9       3.1       6.2       7.3         40%       40%       42.5       14.6       18.2       18.9       20.7       1.2	[41]	Conventional		Cement	Silica fume and		3 D	7 D	14 D	·			1 D	7 D	28 D	Strength
10%       41.5       53.2       57.5       66.9       3.1       6.2       7.3         20%       33.1       42.5       53.3       59.7       2.5       7.3         40%       14.6       18.2       18.9       20.7       1.2       1.2       1.9       4.7         Conventional concrete       0%       7.9       23.2       30.0       - <td< td=""><td></td><td>concrete</td><td>%0</td><td></td><td>metakaolin</td><td></td><td>39.3</td><td>46.1</td><td>54.7</td><td>59.4</td><td></td><td></td><td>3.3</td><td>6.3</td><td>8.9</td><td>increased</td></td<>		concrete	%0		metakaolin		39.3	46.1	54.7	59.4			3.3	6.3	8.9	increased
20%       33.1       42.5       53.3       59.7       2.5       5.5       7.2         40%       14.6       18.2       18.9       20.7       1.2       1.2       1.9       4.7         Conventional concrete       0%       7.9       23.2       30.0       -<			10%				41.5	53.2		6.99			3.1	6.2	7.3	
40%     14.6     18.2     18.9     20.7     1.2     1.9     4.7       Conventional concrete     0.65     -     10     70     28 D     -			%07				33.1	42.5		59.7			2.5	5.5	7.2	
Conventional         Cement         —         0.55 —         1D         7D         28 D         —         —           concrete         0%         7.9         23.2         30.0         — <td></td> <td></td> <td><b>40%</b></td> <td></td> <td></td> <td></td> <td>14.6</td> <td>18.2</td> <td>18.9</td> <td>20.7</td> <td></td> <td></td> <td>1.2</td> <td>1.9</td> <td>4.7</td> <td></td>			<b>40%</b>				14.6	18.2	18.9	20.7			1.2	1.9	4.7	
0%       7.9       23.2       30.0         5%       7.3       21.2       33.3         10%       6.8       19.6       25.3         15%       6.0       17.0       23.1         20%       5.8       14.7       15.6	[99]	Conventional		Cement	1		1 D	7 D	28 D	I			I			Strength
7.3 21.2 6.8 19.6 6.0 17.0 5.8 14.7		concrete	%0				7.9	23.2	30.0							decreased
6.8 19.6 6.0 17.0 5.8 14.7			2%				7.3	21.2	33.3							
6.0 17.0			10%				8.9	19.6	25.3							
5.8 14.7			15%				0.9	17.0	23.1							
			20%				5.8	14.7	15.6							

Fable 1: Continued

Reference	Reference Concrete type Sewage sludge ash	Sewage sludge ash	Sewage Concrete sludge ash ingredient	Other materials W/C Slump Compression strength (MPa) (mm)	W/C Slump (mm)	Compressior	n strength (MPa)	Split tensile strength (MPa)	Flexure strength (MPa)	Remarks
[28]	Conventional		Cement	ı	0.45 —	14 D	28 D	ı	28 D	Strength
	concrete	%0				26	37		0.9	decreased
		2%				25	35		5.1	
		10%				24	33		4.9	
		20%				22	29		4.2	
[51]	Conventional		Additive	ı	0.65	7 D	28 D	ı	ı	Strength
	concrete	%0			180	13.6	16.7			decreased
		2%			125	17.9	25.0			
		10%			30	23.0	28.8			

Note: W/C = water/cement ratio, D = curing days.

ash in both curing procedures, the concrete has stronger properties than regular concrete. After the hydration process in the cement took place, Ca(OH)2 and CSH were produced. The presence of sewage sludge ash in the concrete mixture functions as a pozzolanic material and reacts with these byproducts. The specimen's strength might increase as a result of this interaction. The development of CSH gel was accelerated by the presence of adequate moisture caused by the curing procedure during the hydration phase [55]. The tensile strength of concrete mixes using sawdust ash as a partial replacement for cement was investigated at 7 and 28 days. A reduction in the tensile strength with an increase in the saw dust ash percentage was observed; however, it was less noticeable than a decrease in the CPS. After 7 days, it was seen that the difference in the strength between the blended cement concrete and control mixtures increased. After 28 days, the blended cement concrete mixes with replacement percentages of up to 25% exhibited tensile capacities up to 90% of the strength of the control mixtures [73]. Tensile strength was significantly and just slightly affected by the use of metakaolin and silica fume, respectively. The improved compressive and tensile strength was attained by using the ideal replacement percentage, 15% mixed pozzolan (7.5% metakaolin and 7.5% silica fume) [74]. The optimal dosage of silicon carbide whiskers was 0.1% (by wt), increasing the tensile strength of cement-based materials [75]. Kumar et al. have also shown that the strength significantly decreased at replacement levels of 5, 10, and 15% when 150 µm of smaller sewage sludge ash was replaced compared to concrete samples made with 150 µm of sewage sludge and ash that were reduced to 75 µm [50]. It is possible to conclude that sewage sludge ash with decreasing particle size (below 75 µm) has increased reactivity and, therefore, more efficient strength qualities than sewage sludge ash with greater particle size (150 µm).

#### 3.3 Flexural strength

Figure 7 shows the flexural strength with various replacements of ash and sewage sludge. As opposed to the reference mixes, it should be emphasized that incorporating sewage sludge ash increased the flexural capacity. The flexural strength of the concrete made from sewage sludge and ash is virtually identical to the findings for CPS. When compared to air-cured concrete, the results for sewage sludge and ash concrete were greatest when they were water-cured. This demonstrates why an air-curing

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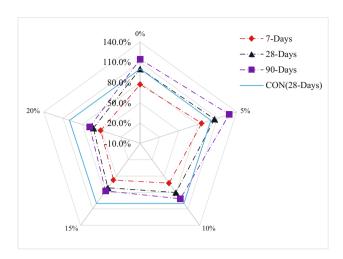


Figure 5: CPS and age relationship [55].

regime is inferior to one that uses water. The best way to stop concrete from losing moisture is to water cure continuously, which will improve the hydration process and pozzolanic reaction [55]. Although excessive amounts of sewage sludge ash concrete may decrease flexural strength due to sample's larger porosity and greater water absorption of the concrete, pozzolanic processes in the concrete may impact the increase of flexural strength [76].

When heated to a medium-high temperature, concrete containing 5% sewage sludge ash had slightly greater flexural strength than the control sample at ages 7 and 28 days. At 7 and 28 days after curing, respectively, the flexural strength of 5% sewage sludge ash heated to a medium-

high temperature was 5.64 and 7.91 MPa. In comparison to other samples, the flexural strength of concrete with a 15% replacement of sewage sludge ash is the lowest. When compared to control samples at age 28 days, the flexural strength of the 15% sewage sludge ash concrete was 39.07% lower [64]. The flexural strength of mortar may be increased by adding mineral admixtures like silica fume and metakaolin. Mortar that was blended with 10% silica fume and 20% sewage sludge ash had slightly greater flexural strength than cement mortar. The ideal dose is 10% metakaolin with a 20% sewage sludge ash replacement rate, which results in a flexural strength that is 28.32% more than cement mortar [41]. The source and processing method of sewage sludge ash has a significant impact on the mortar's flexural strength. For instance, when using the same source of sewage sludge ash and a 10% replacement ratio, the mortar made by sewage sludge ash calcined at 700°C has a 28-day flexural strength, which is greater than that of the mortar prepared by sewage sludge ash calcined at 600°C [78]. Because of the coarse particles, Pavlik et al. [79] discovered that the pozzolanic activity of sewage sludge ash was low.

# 3.4 Elasticity modulus

It is crucial to analyze the results of the modulus of elasticity test since they will show the stiffness properties of

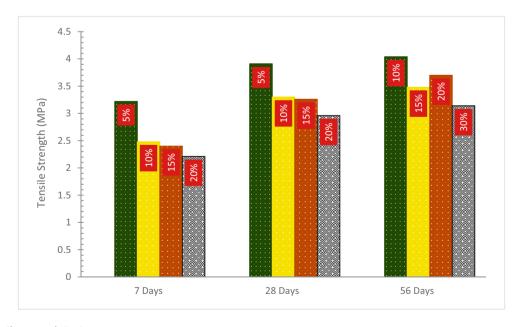


Figure 6: Tensile strength [50].



Figure 7: Flexural strength [50,55,58,77].

the specimen material that exhibits elastic behavior. Figure 8 depicts the association between the addition of sewage sludge ash and the modulus of elasticity of the specimen at 28 days. The findings show that adding concrete with a 5% proportion of sewage sludge ash increases the concrete's rigidity. Additionally, the results demonstrate that the value of the elastic modulus decreased as the amount of sewage sludge and ash concrete increased. This implies that the stiffness of the sewage sludge ash concrete would increase with lower percentages of the sewage sludge ash. Additionally, specimens with water curing had better results than those with air curing [55]. The stiffness of the aggregate, cement paste, and concrete's compactness all affect the material's modulus of elasticity [80].

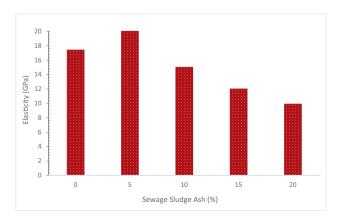


Figure 8: Elastic modulus [55].

A study revealed that the pozzolanic interactions between the binders (ordinary Portland cement [OPC] and wheat straw ash) and continuous hydration caused by the increase in the wheat straw ash concentration increased the elastic modulus of the concrete. According to Katman et al. [81], the potential fine-tuning of the concrete pores may be the cause of the improvement in the elastic modulus of concrete with the buildup of wheat straw ash. This helped the aggregates and the binder, such as wheat straw ash and cement, form stronger interfacial connections [82]. The modulus of elasticity reduced when 150 µm of sewage sludge ash was replaced but it was comparable to the control mix in samples containing 75 µm of downsize sewage sludge ash. The addition of sewage sludge ash resulted in a decrease in elasticity owing to pore structure de-densification. The value of the modulus of elasticity decreases linearly as the sewage sludge ash concentration increases [50].

#### 3.5 Impact strength

Figure 9 displays the impact resistance and energy absorption with varying amounts of sewage sludge ash ranging from 0 to 40%. It should be noticed that the impact resistance and absorbed energy increase until the ash weight rate approaches 10%. This is due to the sludge ash's possible pozzolanic activity, which reduces the CPS of the concrete early on and increases it subsequently. Thus, unreinforced concrete might be used with 10% sludge ash concrete. Additionally, 20% sludge ash concrete might

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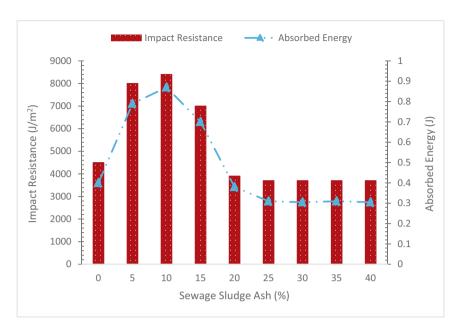


Figure 9: Impact resistance and absorbed energy [72].

be used for the non-structure concrete. For CPS and workability, replacing cement with 30% sludge ash is inappropriate [60].

According to Gonen, when waste crumb rubbers were used in regular concrete at a rate of 4%, the specimen's impact resistance was 200% greater than that of the control concrete [83]. Carmichael and Arulraj compared the impact resistance of concrete with and without the addition of 10–50% of nanomaterial particles and found that the concrete with the nanomaterials had higher impact resistance [84]. More than 20% of sewage sludge ash in concrete exhibits impact resistance and practically constant energy absorption, according to Kutuk and Oguz [72]. Figure 10 compares the impact damage morphologies

of standard concrete samples with silica. After being struck, the samples including silica also fractured in half along the impact direction, showing that the inclusion of silica did not improve the concrete specimens' impact damage morphology and that samples blended with silica still experience fragile failures comparable to those of regular concrete.

## 4 Conclusions

The main objective of this article is to discuss the application of sewage sludge in building materials. Novel properties such as slump flow and setting time as well



Figure 10: (a) Reference sample and (b) 2% addition of silica (pozzolanic materials) [85].

as strength properties such as compressive strength, tensile strength, flexural strength, elastic modulus, impact energy, and absorbed energy are the main aspects of this review.

- Setting time increased with the substitution of sewage sludge ash because the pozzolanic process moves more slowly than OPC hydration. The flowability decreased with sewage sludge ash owing to its spongy nature, which absorbed more water and hence less water is accessible for flowability.
- Strength properties improved with sewage sludge ash due to micro-filling voids and pozzolanic activity, which enhanced the binding properties of the paste. However, optimum substitution is crucial as the higher dose decreased the strength due to a deficiency of flowability, which increased compaction energy, leading to additional cavities. Different studies recommend different optimum doses. However, the typical range for the optimum dose of sewage sludge ash is 5–10% (by wt) of cement.
- It can also be observed that the sewage sludge ash decreased or did not improve early-age strength (up to 7 days). However, improvement was observed with late-age strength (beyond 28 days). It is because the pozzolanic reaction advances more gradually than the hydration of cement, which increases in strength with age.
- This review also concludes that the particle size of sewage sludge ash plays an important role in terms of strength. The lower particle size (75 µm) of sewage sludge ash shows better strength properties as compared to the larger particle size (75 µm).

#### 5 Recommendation

- To increase sewage sludge ash's pozzolanic activity, several techniques like heat activation or alkali activation should be used.
- Less information is available on the impact of particle size of sewage sludge ash on concrete strength. Therefore, this review recommends detailed research on the effect of particle size of the sewage sludge ash on concrete strength.
- Although sewage sludge ash enhanced the concrete strength, the concrete showed low tension. A few studies focus on the tensile capacity of concrete, as shown in Table 1. Therefore, this review also recommends detailed research on the tensile capacity of concrete.

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